

STATE OF THE ART OF RESEARCH ON METHODS AND MODELS FOR SEISMIC RISK ASSESSMENT OF CH CONSTRUCTIONS AND SITES

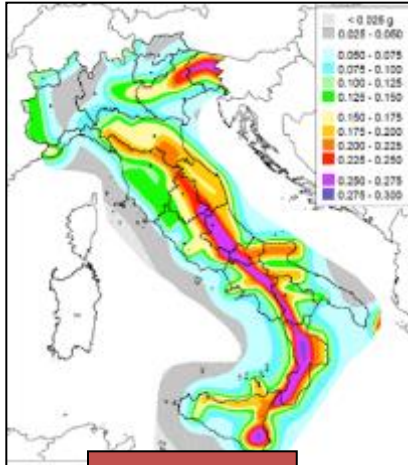
Speaker: Prof. Claudio Modena



INGEGNERIA CIVILE,
EDILE E AMBIENTALE
CIVIL, ARCHITECTURAL AND
ENVIRONMENTAL ENGINEERING



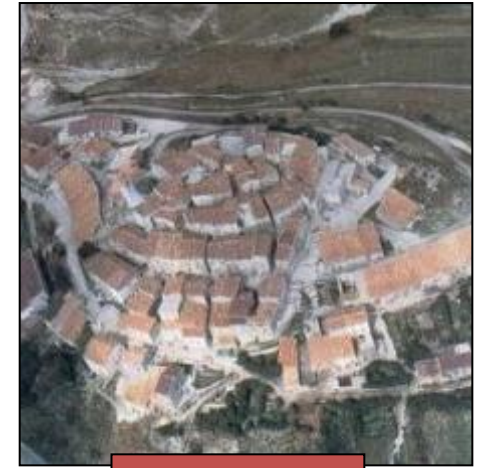
SEISMIC RISK FACTORS: HAZARD + VULNERABILITY + EXPOSURE



HAZARD



VULNERABILITY



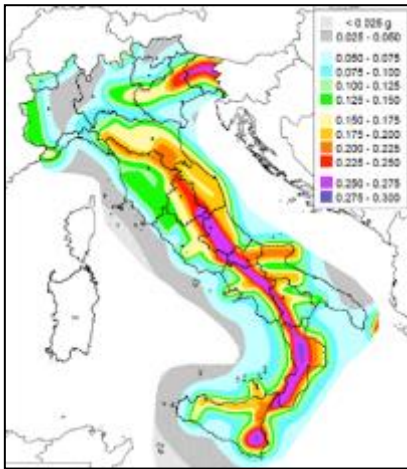
EXPOSURE

Probability of a seismic event of a given magnitude occurring in a certain interval of time.

A structure (building, bridge,..) potential for damage: probability of attaining a given level of damage due to a seismic event of a given intensity.

Losses – of the economy, of human lives, of cultural assets,.. – connected to the damages caused by a seismic event.

SEISMIC RISK MITIGATION: assessment, interventions, plans



HAZARD

VULNERABILITY

EXPOSURE

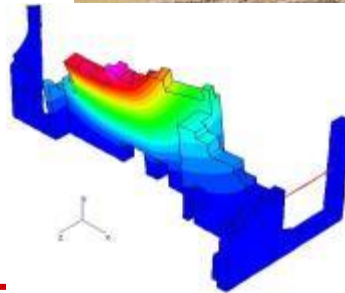
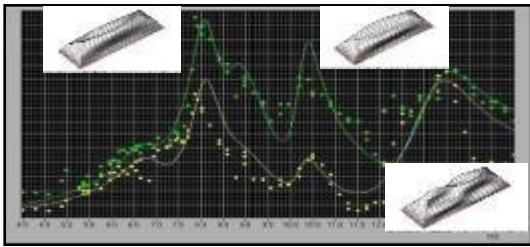
Single building- urban centers/territory

Structure's ruin – archaeological sites

Single bridge- infrastructure networks

Single artistic object – museums

SEISMIC RISK MITIGATION: assessment - interventions



MAJOR EFFORTS SO FAR FOCUSED ON THE DEVELOPMENT OF CRITERIA, METHODOLOGIES AND TECHNIQUES APPLICABLE TO CH CONSTRUCTIONS, ALLOWING FOR:

- **ASSESSING THEIR “REAL” STRUCTURAL SAFETY LEVELS PRIOR AND AFTER INTERVENTIONS**
- **DEFINING “ACCEPTABLE” VALUES OF SUCH LEVELS**
- **ATTAINING THE “TARGET” SAFETY VIA APPROPRIATE REPAIR/STRENGTHENING INTERVENTIONS**

WHILE

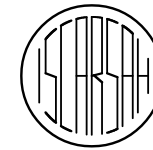
**RECOGNISING THEIR SPECIFIC STATIC/DYNAMIC BEHAVIUR
RESPECTING THEIR INTRINSIC HISTORIC/ARTISTIC VALUES**

RECENT EVOLUTION OF CODES AND GUIDELINES



International Council on
Monuments and Sites

Conseil International
des Monuments et des Sites



RECOMMENDATIONS FOR THE ANALYSIS, CONSERVATION AND STRUCTURAL RESTORATION OF ARCHITECTURAL HERITAGE

Guidelines

1. General criteria
2. Acquisition of data: Information and Investigation
 - 2.2 Historical and architectural investigations
 - 2.3 Investigation of the structure
 - 2.4 Field research and laboratory testing
 - 2.5 Monitoring
3. Structural behaviour
 - 3.1 General aspects
 - 3.2 The structural scheme and damage
 - 3.3 Material characteristics and decay processes
 - 3.4 Actions on the structure and the materials
4. Diagnosis and safety evaluation
 - 4.1 General aspects
 - 4.2 Identification of the causes (diagnosis)
 - 4.3 Safety evaluation
 - 4.3.1 The problem of safety evaluation
 - 4.3.2 Historical analysis
 - 4.3.3 Qualitative analysis
 - 4.3.4 The quantitative analytical approach
 - 4.3.5 The experimental approach
 - 4.4 Judgement on safety
5. Decisions on interventions - The Explanatory Report

RECENT EVOLUTION OF CODES AND GUIDELINES



GUIDELINES FOR THE ASSESSMENT AND THE REDUCTION OF SEISMIC RISK OF CULTURAL HERITAGE

- CHAP. 1: OBJECT OF THE GUIDELINES
- CHAP. 2: SAFETY AND CONSERVATION REQUIREMENTS
- CHAP. 3: SEISMIC ACTION
- CHAP. 4: BUILDING KNOWLEDGE
- CHAP. 5: MODELS FOR SEISMIC SAFETY ASSESSMENT
- CHAP. 6: SEISMIC IMPROVEMENT AND INTERVENTION TECHNIQUES CRITERIA
- CHAP. 7: RESUME OF THE PROCESS

ITALIAN GUIDELINES



interventions designed to "**improve**", not necessarily to "**retrofit**"
the structural performance of CH constructions

Multidisciplinary decision process

based on

both structural analysis and quantitative evaluations

ASSESSMENT - IMPROVEMENT ↔ **VERIFICATION - RETROFITTING**

Italian Guidelines – § 4 – Building knowledge

To carry out the structural analyses, it is necessary to gain **proper knowledge** by means of surveys, historical researches, in-situ and laboratory tests:



BUILDING GEOMETRY



geometry, particular elements (such as chimneys, niches, etc), crack pattern & out of plumbs

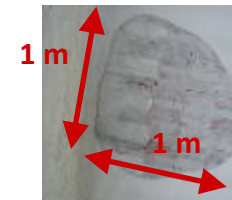
- by means of surveys

CONSTRUCTIVE DETAILS



connections, lintels, elements to counteract thrusts, vulnerable elements, masonry typology

- limited *in situ inspection*
- extended & comprehensive *in situ inspection*



MATERIAL PROPERTIES



particularly aimed at the mechanical characterization of masonry, through inspections, NDT, MDT & DT

- limited *in situ testing* (inspections)
- extended *in situ testing* (MDT & NDT)
- comprehensive *in situ testing* (DT)



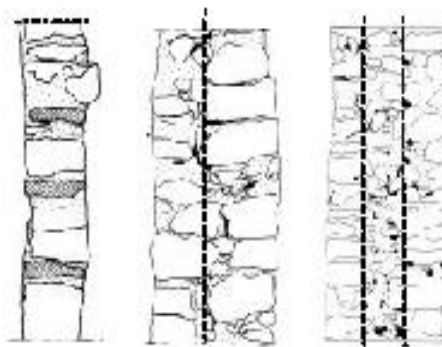
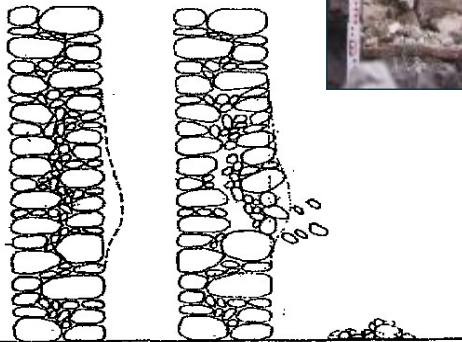
Italian Guidelines – § 4 – Building knowledge

It is possible to refer to abaci for the evaluation of the quality and bearing capacity of **masonry typologies**

Heterogeneous masonry built up with poor materials, presence of voids, irregularities, multi-leaf sections, absence of connections



Out-of-plane brittle collapses



Tipologie		
TIPOLOGIA COSTRUTTIVA	Presenza vuoti	
	Due paramenti accostati	
	Due paramenti sovrapposti	
	Un paramento	
GRANIGLIA	Presenza vuoti	
FRASCO DI ARABOVI (vuoti di grana)	Presenza vuoti	
TRUSSI	Presenza vuoti	

APPARECCHIATURA	Impugnatura	
	Comi sovrapposti	
	Comi orizzontali	
RICORSI	Presenza	
	Assenza	
ZEPPE	Presenza	
	Assenza	

CATALOGO DELLE MURATURE STORICHE		ORIENTAMENTO
CATTELVESTERE	MURATURA DI PIETRA GREZZA	TAV. 01
ELINDI	RAPPRESENTAZIONE SCHEMATICA	
	almeno tre impugnature di pietre sovrapposte in senso orizzontale (C. 15-20/21/22)	
SEZIONI VERTICALI	PARAMETRO ESTERNO	
Sezioni delle pareti: 40, 30, 20 cm	Disposizione degli elementi nel paramento	
Disposizione degli elementi nelle pareti: 40 cm, 30 cm, 20 cm	Aspetto delle pareti: impugnature di pietre sovrapposte in senso orizzontale	
Impugnature di pietre sovrapposte in senso orizzontale	Impugnature di pietre sovrapposte in senso orizzontale	
Impugnature di pietre sovrapposte in senso orizzontale	Impugnature di pietre sovrapposte in senso orizzontale	

CATALOGO DELLE MURATURE STORICHE		ORIENTAMENTO
CATTELVESTERE	SEMI-SEMI FOTOGRAFICHE	TAV. 02
<p>Le pareti sono in pietra grezza con impugnature di pietre sovrapposte in senso orizzontale.</p> <p>Le pareti sono in pietra grezza con impugnature di pietre sovrapposte in senso orizzontale.</p> <p>Le pareti sono in pietra grezza con impugnature di pietre sovrapposte in senso orizzontale.</p>		

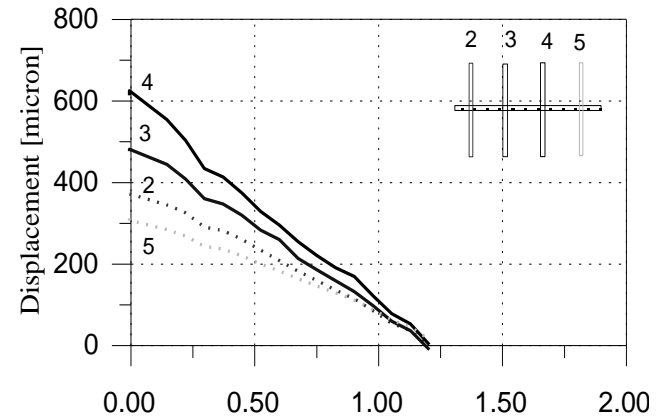
Survey forms: frequent local masonry typologies

Examples of investigation techniques to control the efficiency of repairs

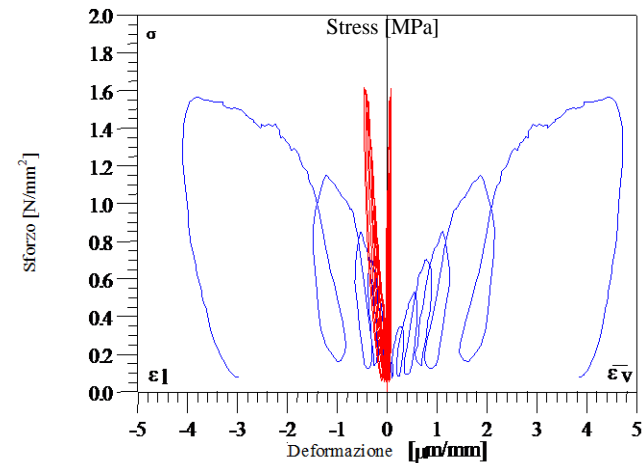
Minor destructive tests (MDT): Flat jack test



Single flat-jack test (detection of state of stress) carried out at the Monza Tower (Binda, 1998)

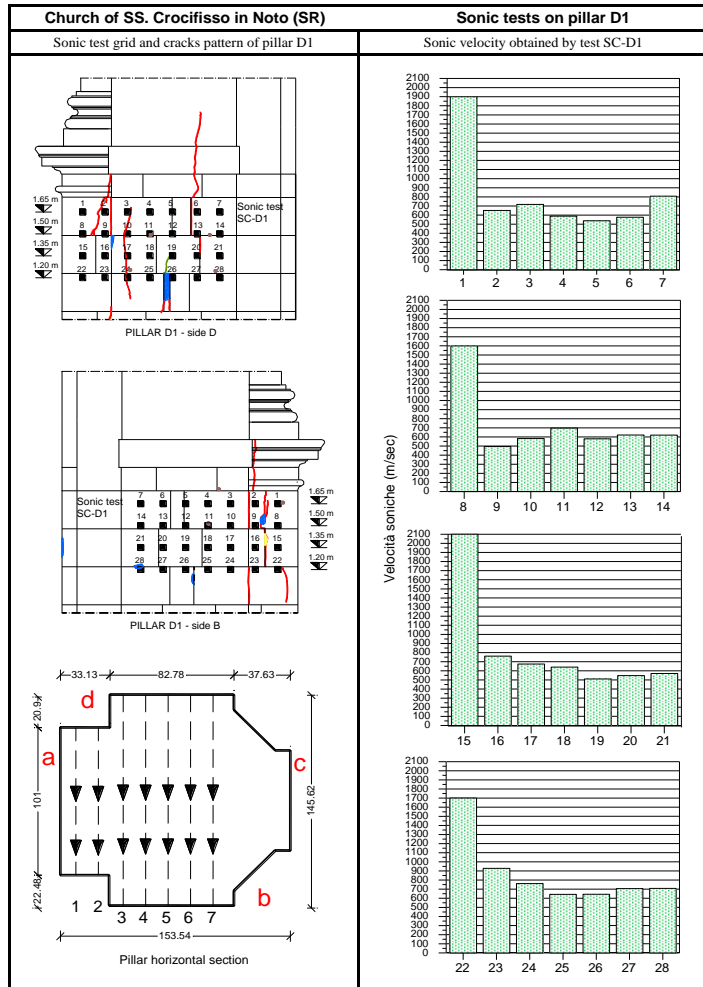


Double flat-jack test (stress-strain behaviour) on West side of the Monza Tower (Binda, 1998)



Examples of investigation techniques for knowledge and control of repairs

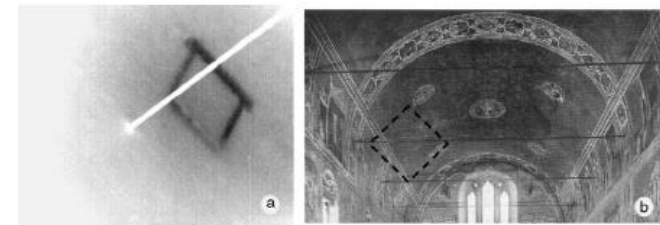
Non destructive tests (NDT): Sonic test



Thermovision



Investigation on hidden steel tie rods



Detention of a modified opening

Examples of investigation techniques for knowledge and control of repairs

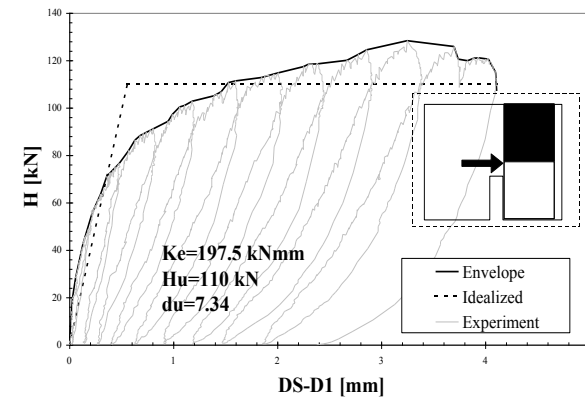
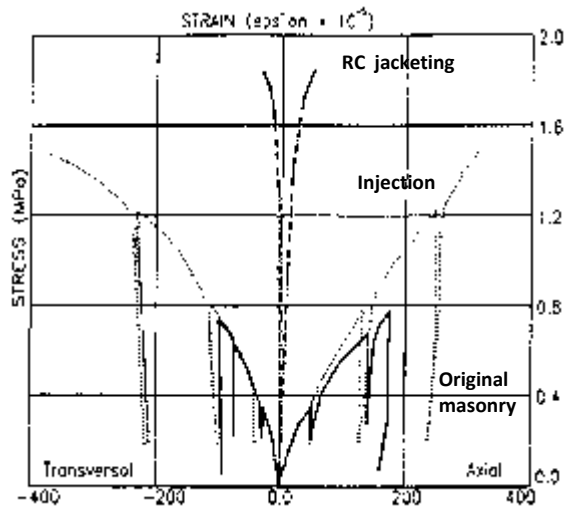
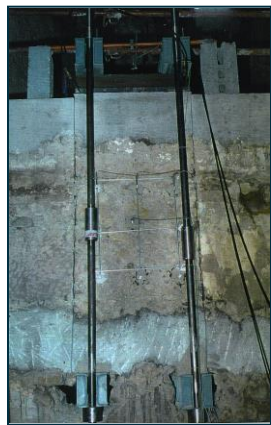
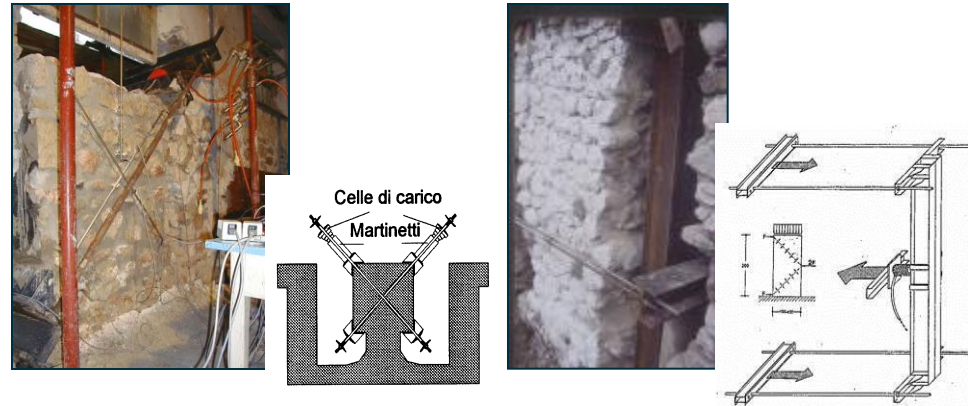
Destructive tests (DT) realised before and after strengthening intervention

Mechanical characteristics for vertical actions



(Bettio, Modena 1993)

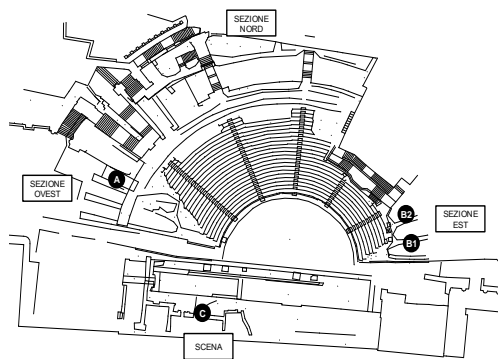
Mechanical characteristics for horizontal and vertical actions



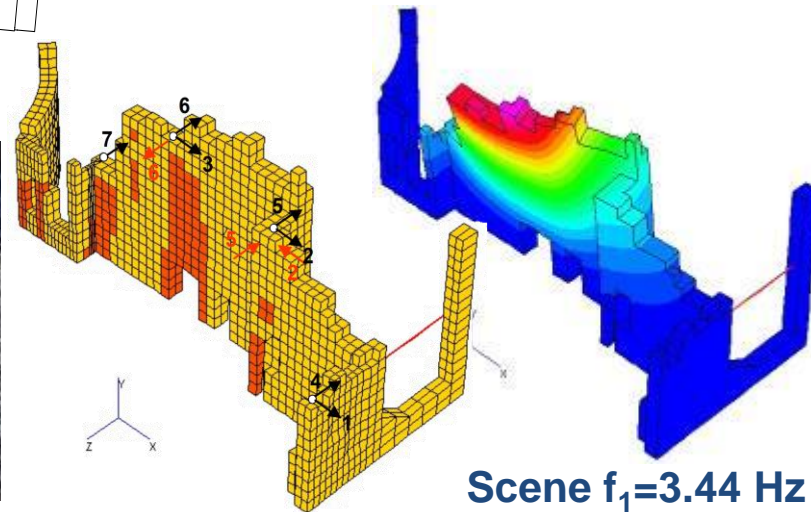
Examples of modal identification for knowledge

Roman Theatre (Verona)

- shock test
- stepped sine test
- modal identification



Proprietà 1	E	1.2E+03	MPa
	ρ	1.3E+03	kg/m ³
Proprietà 2	E	3.2E+03	MPa
	ρ	1.3E+03	kg/m ³
Proprietà 3	E	1.2E+03	MPa
	ρ	1.5E+03	kg/m ³



Scene $f_1=3.44$ Hz

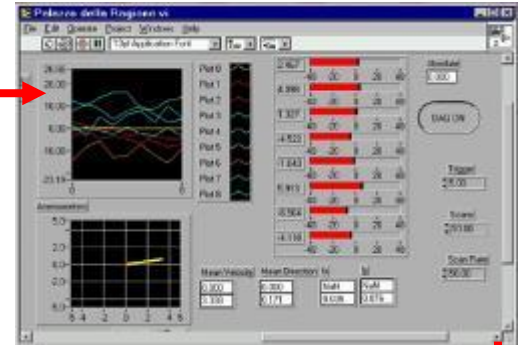
Examples of monitoring systems for knowledge and control of repairs

Palazzo della Ragione (Padova, XIII-XV C.)

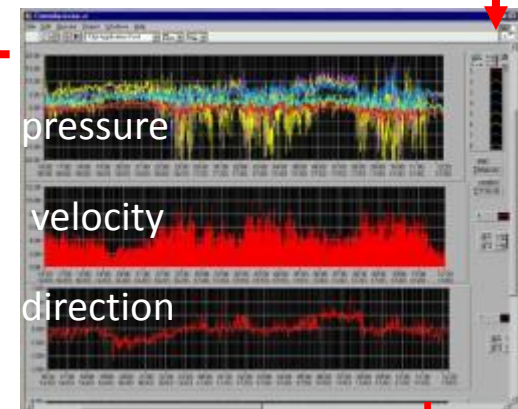


Idea:

- wind effects monitoring
- stepped sine test
- FE modeling
- modal identification
- www data management



Data acquisition

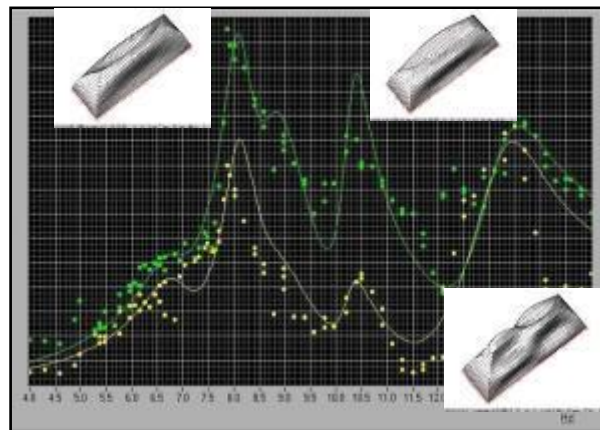


Modal analysis

www database



Web monitoring home page



Italian Guidelines – § 5 – Models for seismic safety assessment

Modelling for typologies

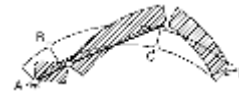
Simplified tools (for **LV1** level of assessment) are given for the analysis and modelling of buildings that can be ascribed to specific constructive typologies:

Palaces, villas, and other buildings with in-between horizontal floors and load bearing wall

Churches, oratories, and other buildings with large rooms without in-between floors

Towers, bell-towers and other buildings with main vertical length

Masonry bridges, triumphal arches and other arch and vaulted structures



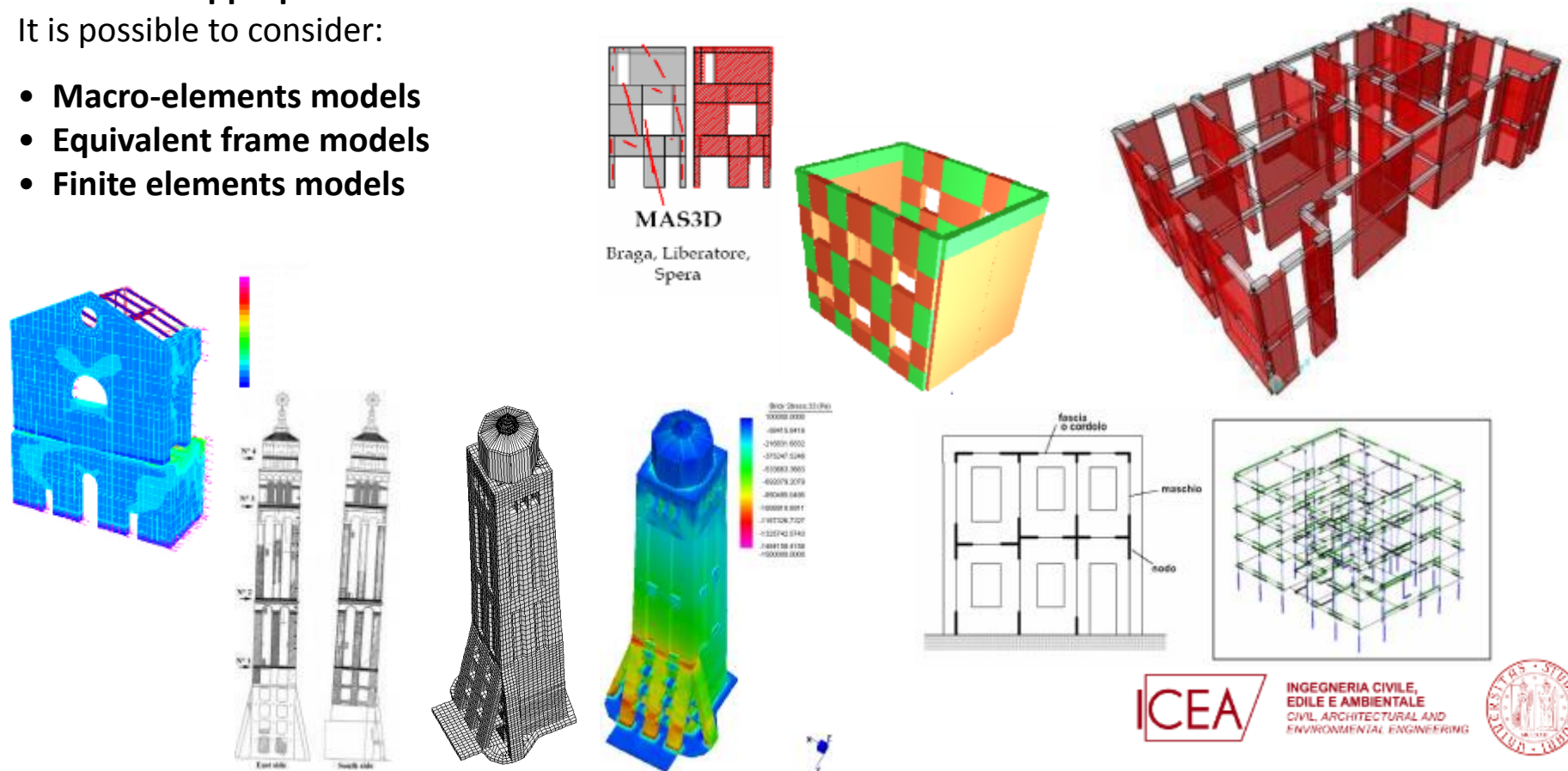
Italian Guidelines – § 5 – Models for seismic safety assessment

Structural modelling and seismic analysis methods

For existing masonry buildings it is possible to consider **various analysis methods**, according to the considered **appropriate model** which describe the structure and its seismic behaviour.

It is possible to consider:

- Macro-elements models
- Equivalent frame models
- Finite elements models



EXTENSIVE EXPERIMENTAL RESEARCHES TO VALIDATE REPAIR/STRENGTHENING TECHNIQUES

Shaking table tests on out-of-plane behavior of single structural elements: stone masonry wall



Unstrengthened
condition
0.25g



Strengthened
using ties
0.45g

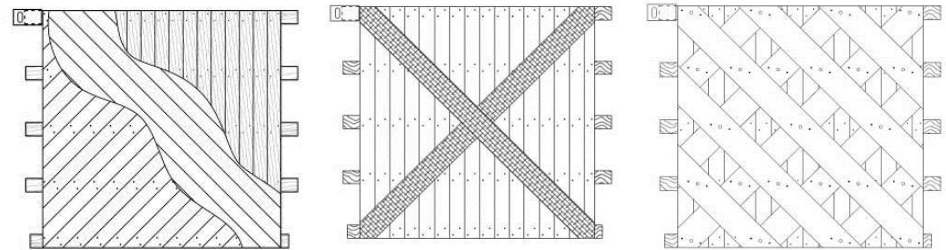


Strengthened
using injection
0.60g



Strengthened using ties
and injection
0.75g

- **Different strengthening systems** (plankings, diagonals, nets, ..) and materials (wood, earth, FRP, Natural fibres) applied at the extrados, for a total of **35 laboratory tests**
- High performance obtained for wooden **planking** (45°, single or double) both for strength and deformation capacity
- The **shear stiffness** of the joist ceiling is principally influenced by the planking thickness
- The **shear capacity** of the floors is linearly related with the strength of the fasteners
- Proper double **planking** provides stiffness capable to redistribute horizontal loads to bearing walls, comparable to the effect of more modern floors



Strengthening materials: CFRP, SRP, SRG, BTRM



SEISMIC RISK MITIGATION: plans /strategies - territorial level

- Historic centres



- Industrial areas



- Infrastructures



- Archaeological sites



URBAN CENTERS: SIMPLIFIED METHODOLOGIES BASED ON MECHANICAL MODELS

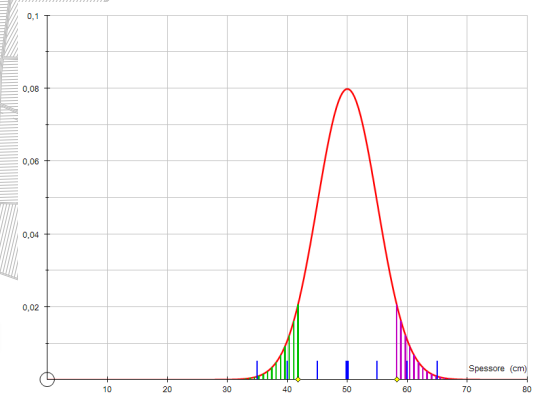
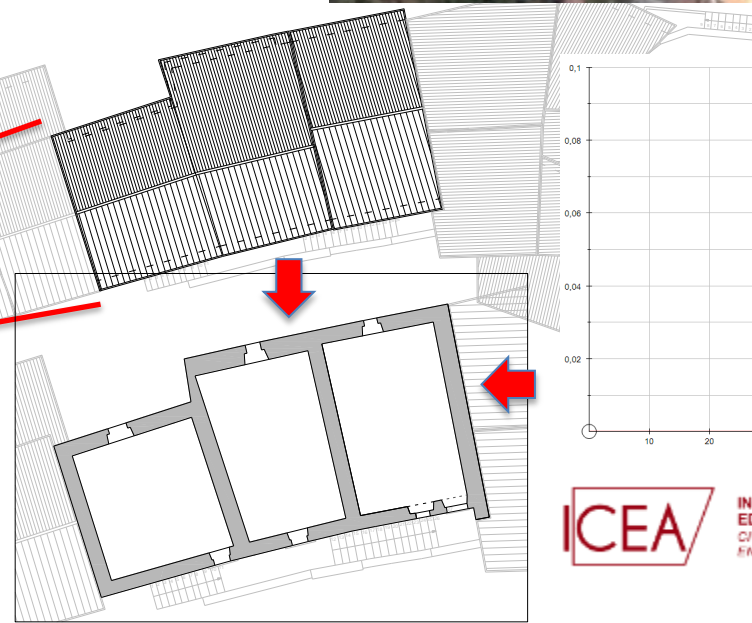
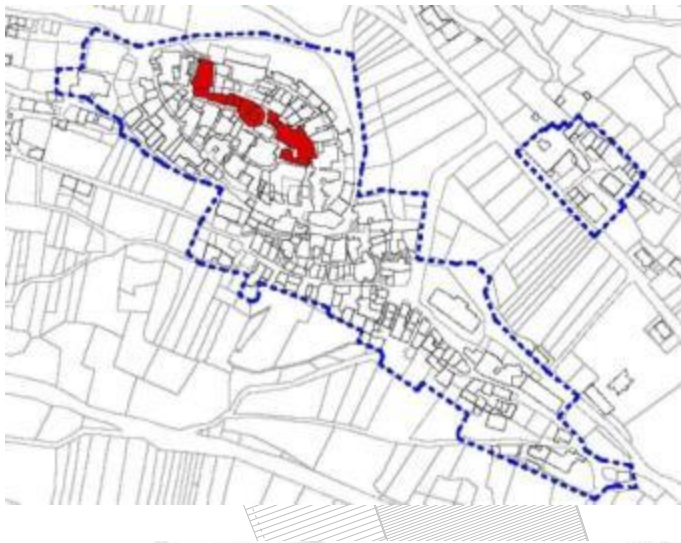
Automatic procedures for the systematic assessment of the existing masonry buildings vulnerability developed by the University of Padova:

Vulnus: global seismic vulnerability analysis (vulnerability assessment and damage probability) of isolated or clustered masonry buildings through different in plane and out of plane mechanisms combinations and qualitative informations.

c-Sisma: local analysis of vulnerability through the application of single kinematic models applied to the more significant macroelements. It also performs the safety analysis according to the Italian regulation



SIMPLIFIED METHODOLOGIES BASED ON REMOTE SENSING DATA PROCESSING



SIMPLIFIED METHODOLOGIES BASED ON MECHANICAL MODELS

SURVEY FORMS

- Scheda di 1° livello di rilevamento Danno, pronto intervento e Agibilità per edifici ordinari nell’Emergenza post-Sismica (AeDES)

- typological and damage in the emergency phase
- physical evaluation of the damage
- conformity to standards analysis



- Scheda di rilievo per la catalogazione delle caratteristiche tipologiche, della vulnerabilità e del danno (PoliMI):

- in site geometrical and damage survey
- qualitative masonry information
- in site and laboratory tests

G.N.D.T. - SCHEDA DI VULNERABILITA' DI 2° LIVELLO (MURATURA)

PARAMETRO	DESCRIZIONE	VALORI	SCHEMI RICAMBI
1	TIPO ED ORGANIZZAZIONE DEL SISTEMA RESISTENTE (G.A.)	G.A. A) Mura separate G.A. B) Cortile a cubo full level G.A. C) Mura perimetrali in corteo G.A. D) Mura con cortili interni	G.A. A) G.A. B) G.A. C) G.A. D)
2	QUALITA' DEL S.T.C.	S.T.C. A) Mura in pietra S.T.C. B) Mura in mattoni S.T.C. C) Mura in blocchi	S.T.C. A) S.T.C. B) S.T.C. C)
3	RESISTENZA CONDIZIONALE	R.C. A) Mura in pietra R.C. B) Mura in mattoni R.C. C) Mura in blocchi	R.C. A) R.C. B) R.C. C)
4	POSIZIONE EDIFICIO FONDAZIONE	F.O. A) Mura in pietra F.O. B) Mura in mattoni F.O. C) Mura in blocchi	F.O. A) F.O. B) F.O. C)
5	ORIZZONTAMENTO	O.R. A) Mura in pietra O.R. B) Mura in mattoni O.R. C) Mura in blocchi	O.R. A) O.R. B) O.R. C)
6	CONFIGURAZIONE PLANIMETRICA	P.L. A) Mura in pietra P.L. B) Mura in mattoni P.L. C) Mura in blocchi	P.L. A) P.L. B) P.L. C)
7	CONFIGURAZIONE IN ELEVAZIONE	E.L. A) Mura in pietra E.L. B) Mura in mattoni E.L. C) Mura in blocchi	E.L. A) E.L. B) E.L. C)
8	COPERTURA	C.O. A) Copert. in legno C.O. B) Copert. in cemento C.O. C) Copert. in metallo C.O. D) Copert. in altri materiali	C.O. A) C.O. B) C.O. C) C.O. D)
9	ALTRA VULNERABILITA'	A.V. A) Mura in pietra A.V. B) Mura in mattoni A.V. C) Mura in blocchi	A.V. A) A.V. B) A.V. C)

- Scheda di vulnerabilità di 2° livello (muratura) – G.N.D.T.:

- sum of factors which define the vulnerability index:
- definition of the vulnerability classes for different parameters
- data quality information assessment

SIMPLIFIED METHODOLOGIES BASED ON MECHANICAL MODELS

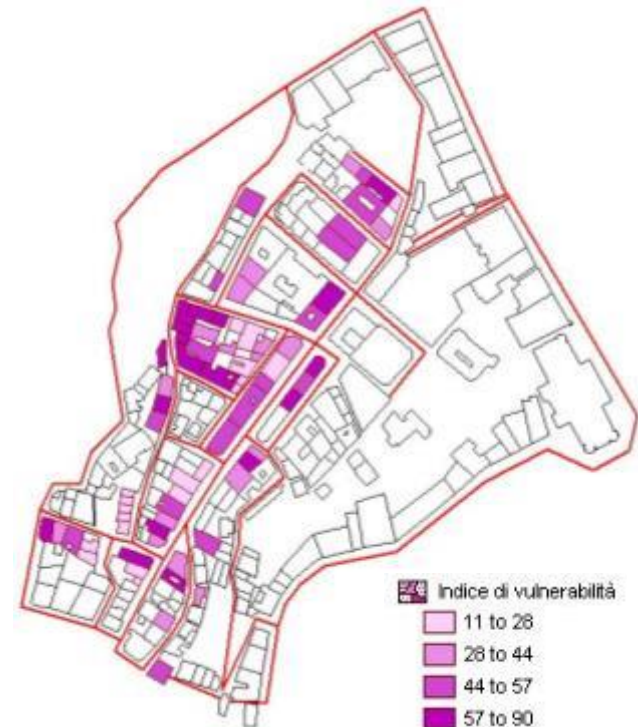
MARVASTO PROJECTS – VALPARAISO (CILE)

Speed seismic vulnerability schedule for the survey of 70 buildings of the Cerro Cordillera in Valparaiso (UNESCO zone):

- definition and classification of 11 parameters;
- weighted and normalized average of the classes scores gives the I_V Vulnerability index.



G.E.M.M. SCHEDA DI VULNERABILITA' SISMICA DI 2° LIVELLO					
Comune Valparaiso - Cerro		Edificio	R.O.	Foto:	
PARAMETRI	Classi	Qual. int.	ELEMENTI DI VALUTAZIONE	SCHEMI E RICHIAM	
1			Norme norme costruttive vigenti (N.T.C.) Categorie ed edifici ed costruzioni a L.R. 1/85 Basi appiccicate in mal e tra due piani consecutivi Edifici con valori sismologici	A B C D	3. Scemmi L. 10/85 1. Osservato (0.5) → 1.00 2. 0.50 → Osservato (1.5) → 0.60 3. Osservato (mass) → 0.30
2			1. Materiali usati 2. Materiali usati 3. Dimensione muratura/colonna/legno 4. Elettro in edificio legno 5. Dimensionamento delle pareti (se)		1. Osservato → 1.00 2. Osservato → 0.50 3. Osservato → 0.20 4. Osservato → 0.10
3			1) Resistenza in aggregato 2) Dimensione pareti 3) Numero di piani n 4) Superficie coperta S 5) Piani specifici pareti p (kg/m²)		1. Osservato → 1.0 2. Non osservato → 0.5 Osservato (0.1) → 0.05 (kg/m²) Osservato (0.5) → 0.05 (kg/m²)
4			1) Fondazioni (percentuale del terreno (%)) 2) Differenza tra quote di (se) 3) Rende (se) 4) Terreno solido non sismologico (SN) 5) Terreno solido sismologico (SN)		E. Osservazione planimetria F. Osservazione in sezione
5			1) Piani sismici (SN) 2) Osservazioni rigide e non collegati 3) Osservazioni deformabili e non collegati 4) Osservazioni deformabili e non collegati		G. Osservazione in sezione
6			Rapporto perimetrale $p = 4 \cdot h \cdot \dots$ Rapporto perimetrale $p = 4 \cdot h \cdot \dots$		H. Osservazione in sezione
7			1. Aspetto o distribuzione massa (%) 2. Distribuzione porosità (%) 3. (se)		I. Osservazione in sezione
8			Rapporto massa $m = \dots$ Rapporto massa $m = \dots$		J. Osservazione in sezione
9			1. Copertura appesa (SN) 2. Calce ai collegamenti (SN)		K. Osservazione in sezione
10			(vedi manuale)		
11			(vedi manuale)		



METHODOLOGY APPLICATION: ITALIAN HISTORIC CENTRES

Montesanto , Roccanolfi, Campi Alto e Castelluccio di Norcia (PG)

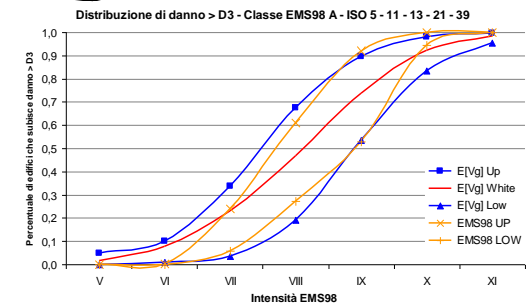
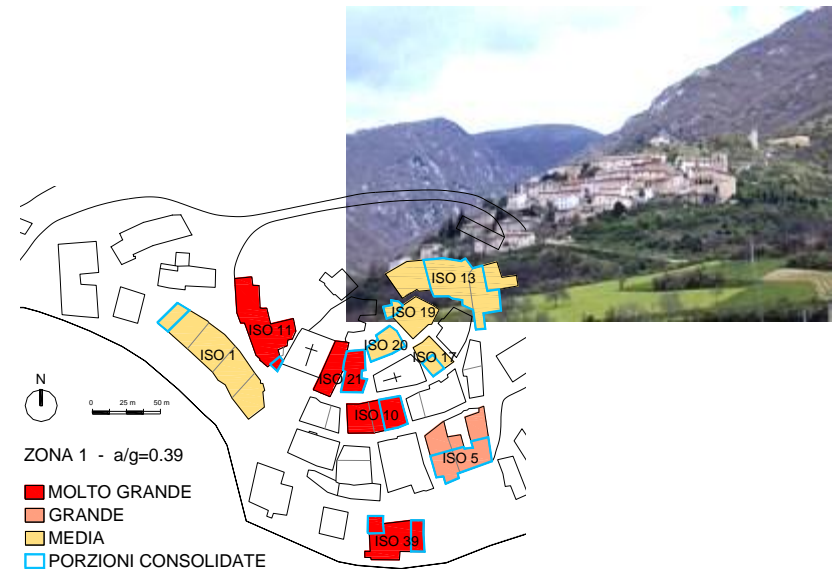
Vittorio Veneto (TV), Campo di Brenzone (VR)

Sulmona (AQ)

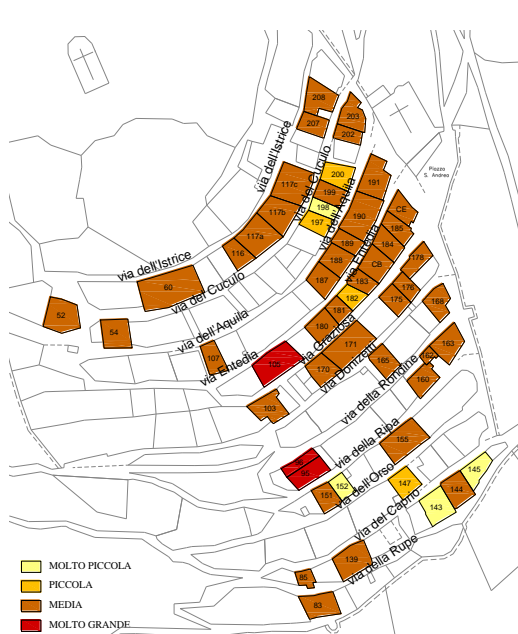
- Complex clustered buildings analysis:
 - considering interaction because of the structural continuities
 - individuation of the structural seismic units (U.S.); global simplified evaluation of the seismic capacity

- Elaboration of useful **post seismic condition tools** for the Protezione Civile and the public administrations for the **reduction of the seismic vulnerability** on an urban scale:

- vulnerability maps
- fragility curves
- damage scene



Linguistic assessment of vulnerability **MEDIUM** ($a/g = 0,32$) and **VERY SMALL** ($a/g = 0,19$)



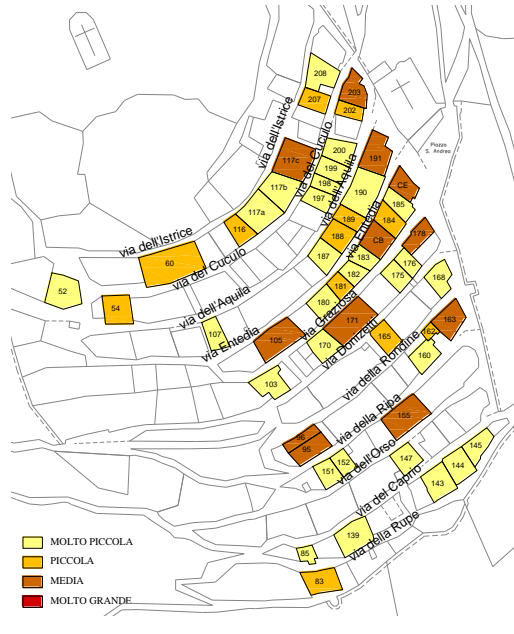
$a/g = 0,32$

- $a/g = 0,32$ (NTC2008)
- $a/g = 0,19$ (storico)

Classe EMS98A:

Classe EMS98B:

Classe EMS98C:



$a/g = 0,19$

⇒ $E[Vg] > 40\%$

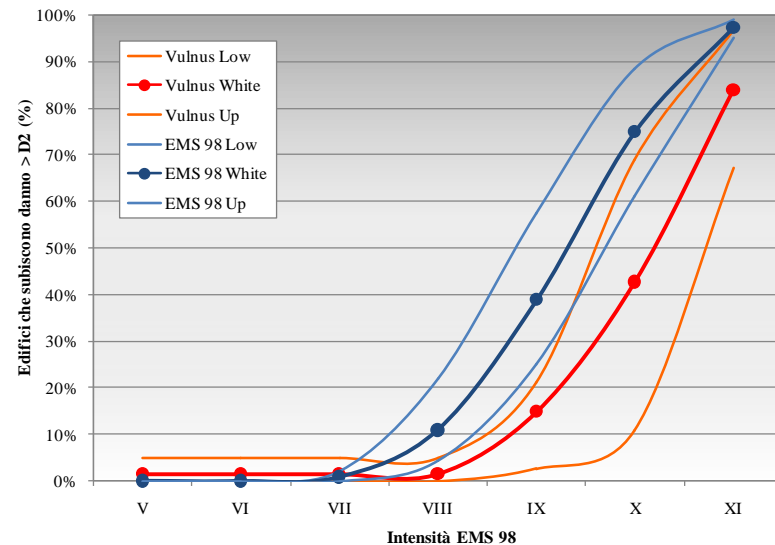
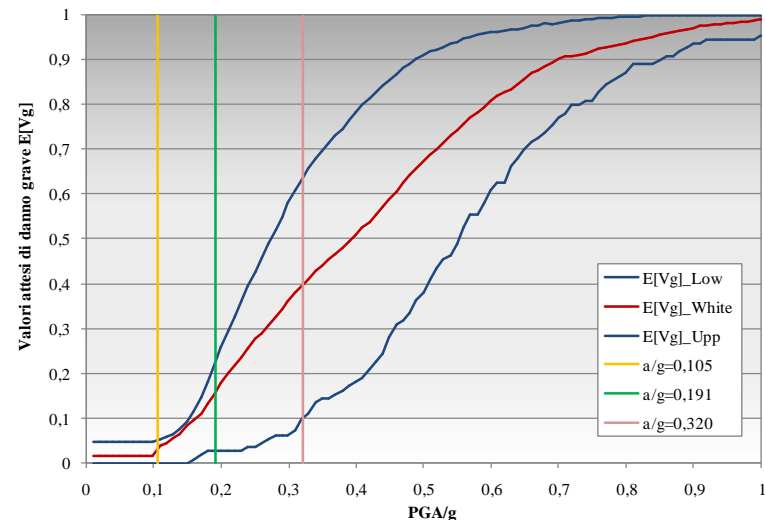
⇒ $E[Vg] \approx 15\%$

0 U.S.

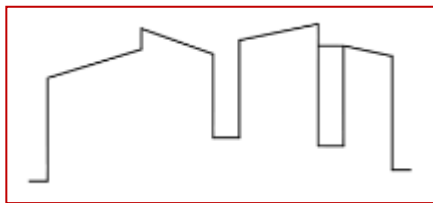
1 U.S.

49 U.S.

CAMPI ALTO

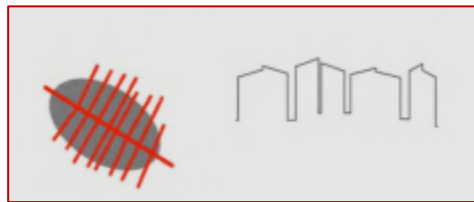


APPLICATION ON DAMAGE SCENARIOS: RECONSTRUCTION PLANS IN ABRUZZO

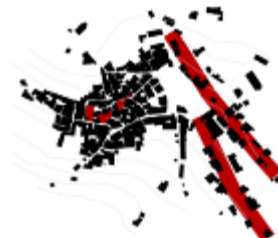


SANTO STEFANO DI SESSANIO

**CASTELVECCHIO
CALVISIO**



CASTEL DEL MONTE



**VILLA SANTA LUCIA DEGLI
ABRUZZI**



UNIVERSITÀ
DEGLI STUDI
DI PADOVA

SETTE AZIONI
PER AGGIORNARE
IL P.T.R.C. adottato

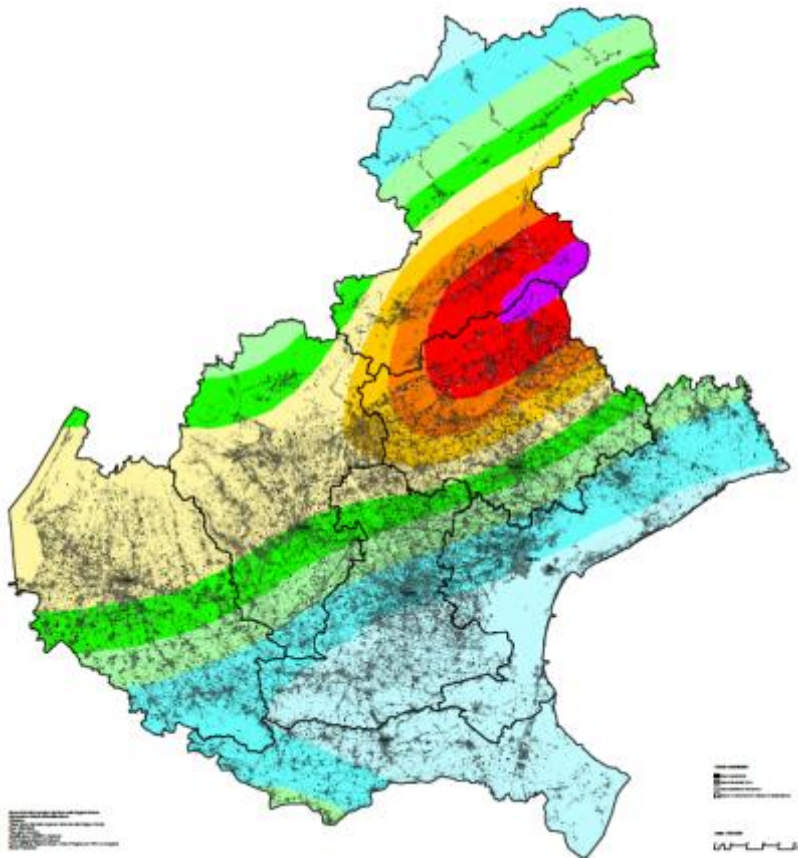


REGIONE DEL VENETO
PTRC
piano territoriale regionale di coordinamento

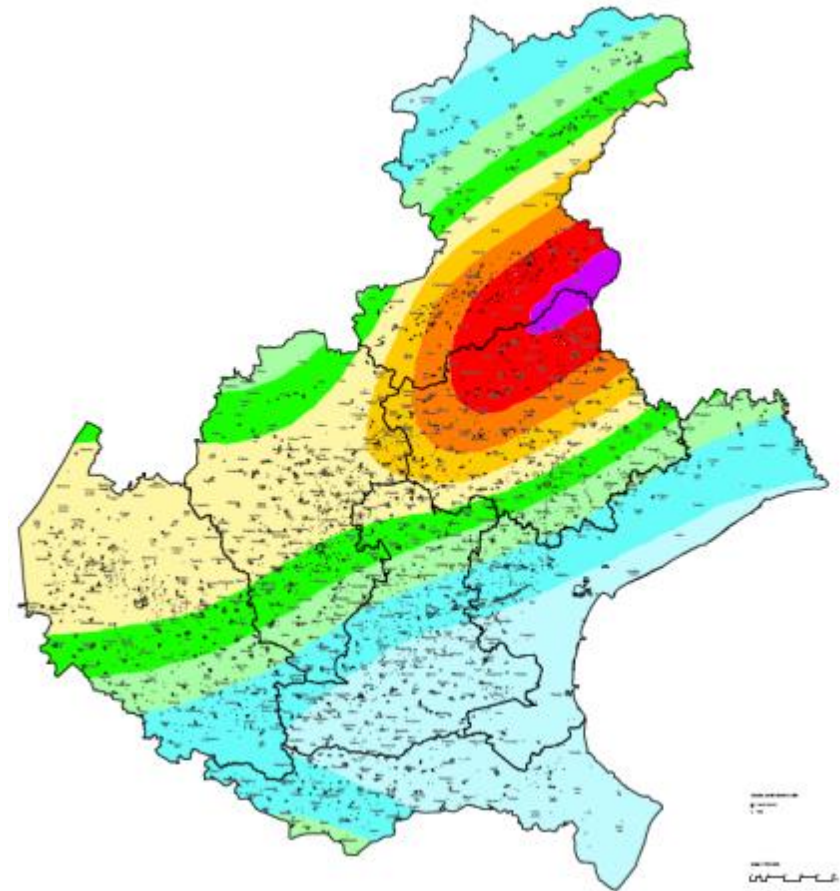
REGIONAL COORDINATION TERRITORIAL PLAN

Agreement for the definition of general criteria for seismic vulnerability assessment of systems (historic and urban centres, industrial areas, infrastructural systems) on a regional scale and for mitigation of seismic risk in areas with a significant hazard

- Development of strategies for seismic risk reduction in the framework of the hierarchical planning system



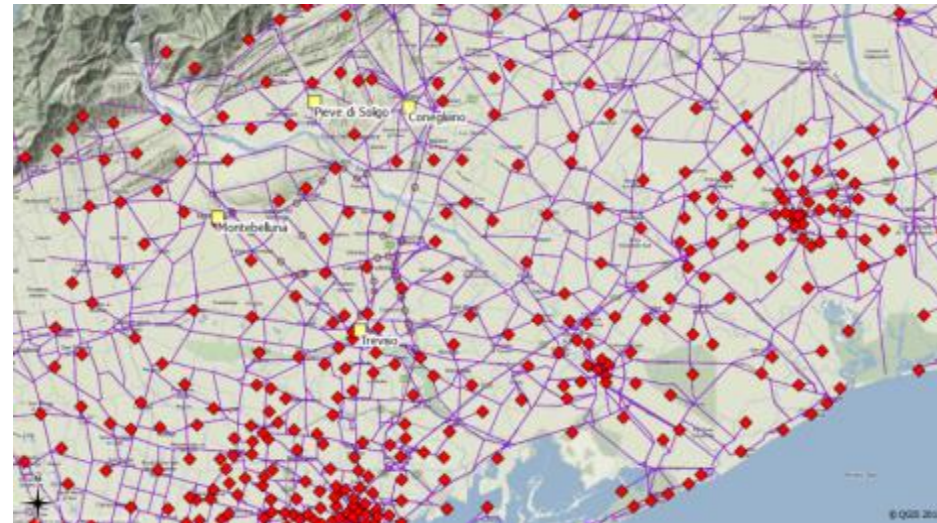
Urban areas and historic centres



Venetian Villas

THE MODELS TO PERFORM SEISMIC RISK ANALYSIS (SRA) FOR BUSINESS AND TRANSPORTATIONS LOSSES

- Hazard
 - Scenario Earthquake
- Vulnerability:
 - Bridge
 - Industrial building
- Risk:
 - Loss of production



THE MODELS TO PERFORM SEISMIC RISK ANALYSIS (SRA) FOR BUSINESS AND TRANSPORTATIONS LOSSES

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A GENERAL LOSS MODEL FOR SEISMIC RISK ANALYSIS

$$Loss = D + I$$

$$D = Bu + Br$$

$$I = Time + Product$$

D: Direct losses

I: Indirect losses

Bu: Direct losses connected to reconstruction cost in buildings

Br: Direct losses in term of bridge reconstruction/rehabilitation cost

Time: The increase in transportation network time can be monetized as: 15.00€/hour

Product: Loss of production due to earthquake damage to industrial facilities

Earthquake scenario

Business interruption

Transportation analysis

Risk as loss of production

Financial aids

Industrial building reconstruction

Risk as transportation delay

Risk as emergency response

Reconstruction cost bridges

Risk

BUSINESS LOSSES - HOW TO DEAL WITH BUSINESS DAMAGE

Earthquakes
scenario

Damage
State

$$P[ds|PGA] = \Phi \left[\frac{1}{\beta_{ds}} \ln \left(\frac{PGA}{PGA_{d,ds}} \right) \right]$$

Residual
functionalit
y

$$RF = f(DS)$$

Residual
productivity

$$Loss = F \times P$$

Residual functionality
model

- Loss of functionality

Economic data

- Loss of Gross Regional Product

Recovery model

- Recovery process

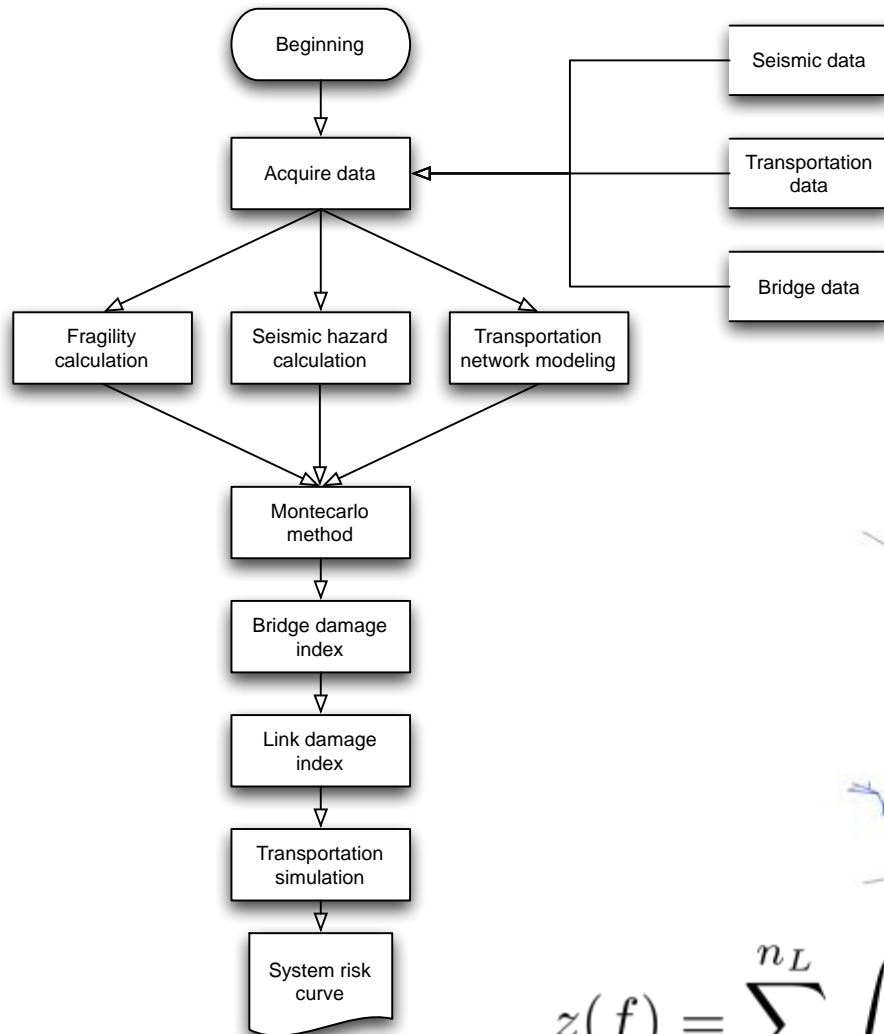
Estimation of
reconstruction cost

- Cost Benefit analysis

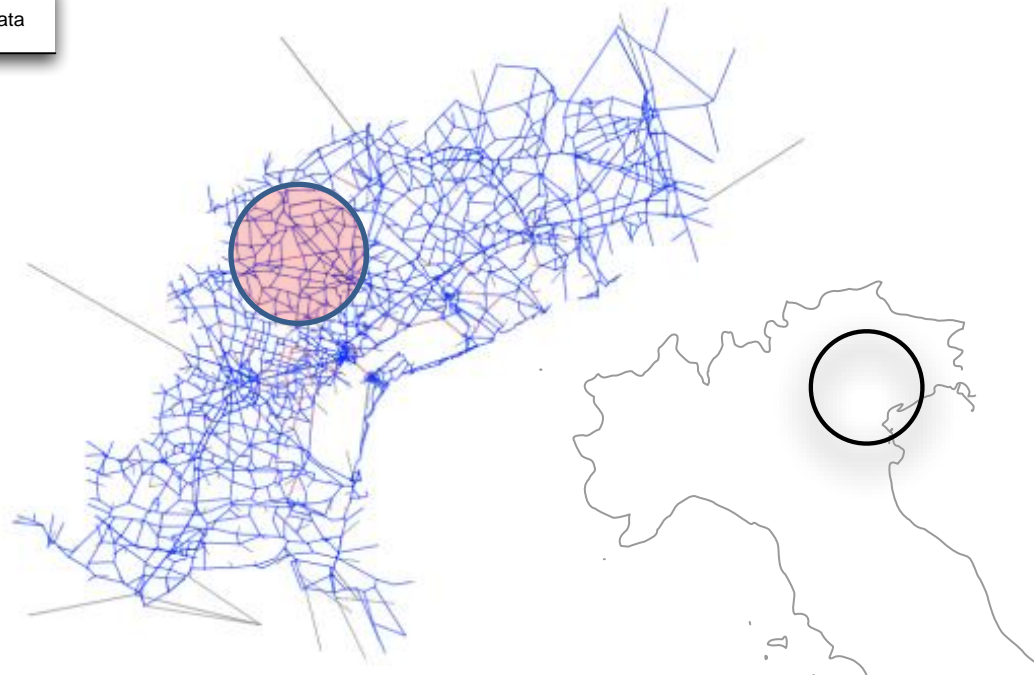
$$BD = F(PR, RF, FR, E)$$

- BD: Business Damage
- PR: Residual Productivity
- RF: Residual Functionality
- FR: Fragility of building
- E: Earthquake

TRANSPORTATION LOSSES - HOW TO MODEL THE TRANSPORTATION NETWORK



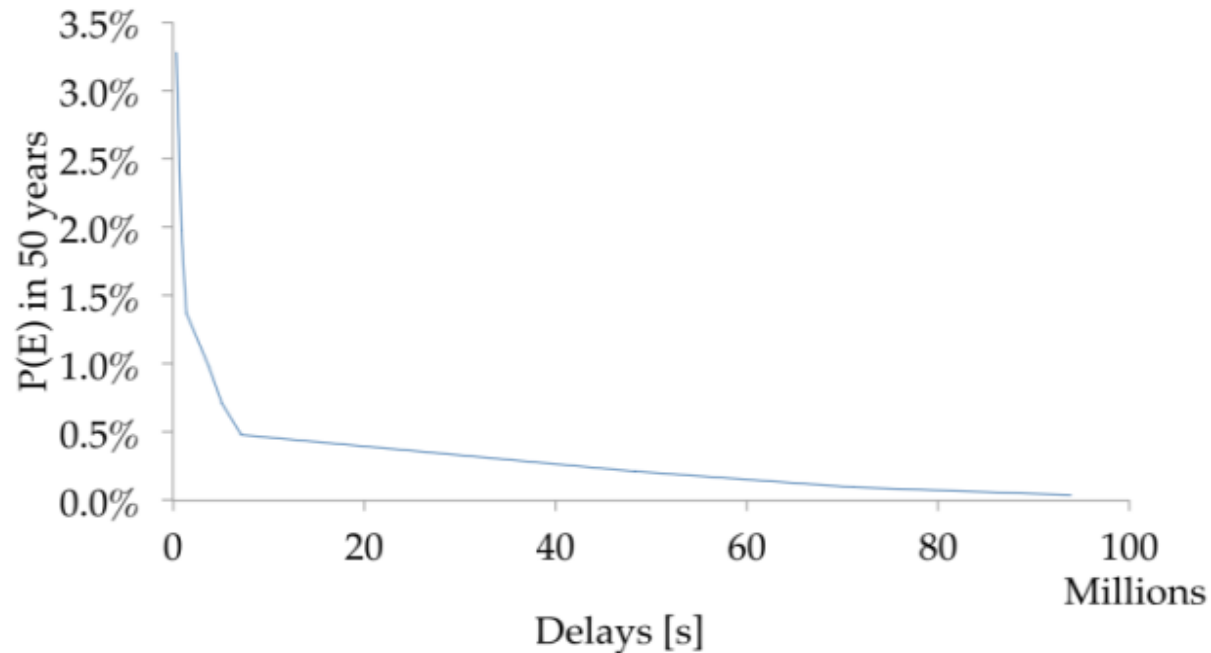
Number of bridges = 41
 Number of links = 11'149
 Number of nodes = 5294
 Spatial extension = 100 km x 100 km



$$z(f) = \sum_{l=0}^{n_L} \int_0^{f_l} c_l(x) dx$$

THE FINAL RISK CURVE FOR INDIRECT LOSSES IN THE TRANSPORTATION

Risk curve



$$T_s = \sum_{l=0}^{n_L} f_{l,s} \cdot \bar{c}_l(f_{l,s}) \forall s$$

$$T_i = \frac{1}{N_{it} MC} \sum_{s=0}^{N_{scn}} T_s \quad \forall i$$

SEISMIC RISK MITIGATION: artistic objects - museums

BRONZO A

OLD SEISMIC ISOLATION



NEW SEISMIC ISOLATION

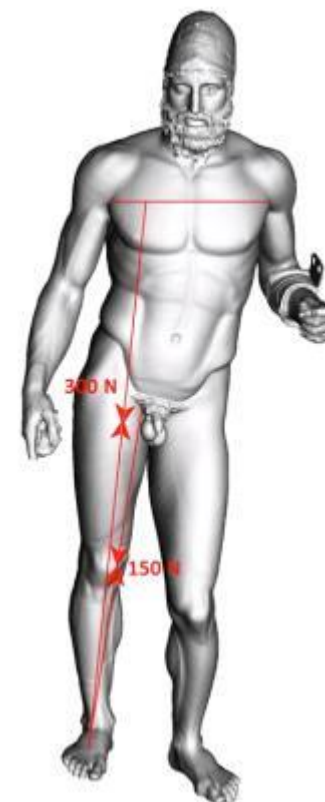


BRONZO B

OLD SEISMIC ISOLATION



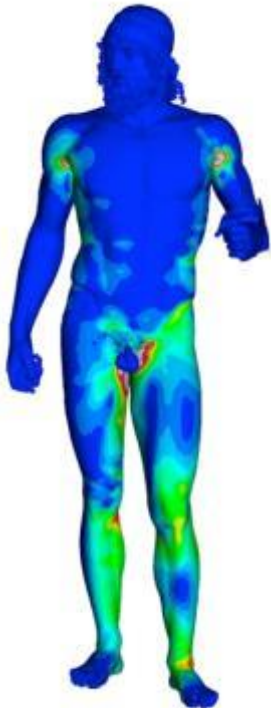
NEW SEISMIC ISOLATION



(I) STRESS DISTRIBUTIONS- VON MISES CRITERION (MPa)

EXTERNAL SURFACE OF THE BRONZO

OLD SEISMIC ISOLATION



NEW SEISMIC ISOLATION



EXTERNAL SURFACE OF THE BRONZO

OLD SEISMIC ISOLATION



NEW SEISMIC ISOLATION



Seismic protection and mitigation measures for artworks

Damage to the collection of the Archaeological Museum in Kobe, Japan (1995)



Museography design is generally conceived so that artworks can be displayed in a variety of manners:

1. works directly laid on the floor (statues)
2. works that are displayed inside bigger display cases
3. works directly laid on the podiums
4. works under a glass bell jar put on the podiums.

Show cases not adequately anchored can **slide**, **rock** and/or **overturn** during earthquake and cause damages to the case itself and its content. On the other hand show cases, even adequately bolted or anchored, can suffer **high acceleration amplifications**, requiring a number of reliable anti-seismic devices (elastic net, clips, etc.) applied to the single item, or group of objects exhibited in the display case.

There are **many techniques available** to reduce potential non-structural earthquake damage:

- providing **base isolation** or seismic shock absorber for standing-alone showcases or large podia (*rolling bearing device, wheels on rails isolators, ...*) ;
- mitigation measures** intended to increase resistance and structural redundancy of the case itself, like using of adequate anchor bolts to provide rigid anchorage to the floor, or bracing to the structural slab.

Anti-seismic devices for lightweight structures

PRINCIPAL CHARACTERISTICS FOR A GENERAL ANTI-SEISMIC DEVICE:

- ability to support gravity loads, both under static and seismic conditions;
- high deformability (or low stiffness) in the horizontal direction under seismic actions;
- appropriate energy dissipation capacity;
- adequate resistance for the horizontal non-seismic actions.

Another important characteristic, although not essential, is:

- ability to re-centering, which allows to have no or negligible residual displacements at the end of the seismic actions.

UNSUITABILITY OF TRADITIONAL ANTI-SEISMIC DEVICES FOR LIGHTWEIGHT STRUCTURES:

ELASTOMERIC DEVICES: they are neither economically advantageous nor, in some cases, technically suitable, because *the bearing function is coupled to the reduction function of horizontal stiffness*

With low values of mass to isolate (< 10 t) in fact, to achieve a sufficiently high natural period of vibration is possible to act on two device's parameters: increase the height or decrease the plan size of device → increase of the slenderness !

IT IS BETTER TO SEPARATE FUNCTIONS INTO:

- **bearing of load**
- **accommodation of horizontal displacement**
- + provision of a restoring force



sliding / rolling bearings
+ dished tracks or auxiliary springs

Anti-seismic devices for lightweight structures

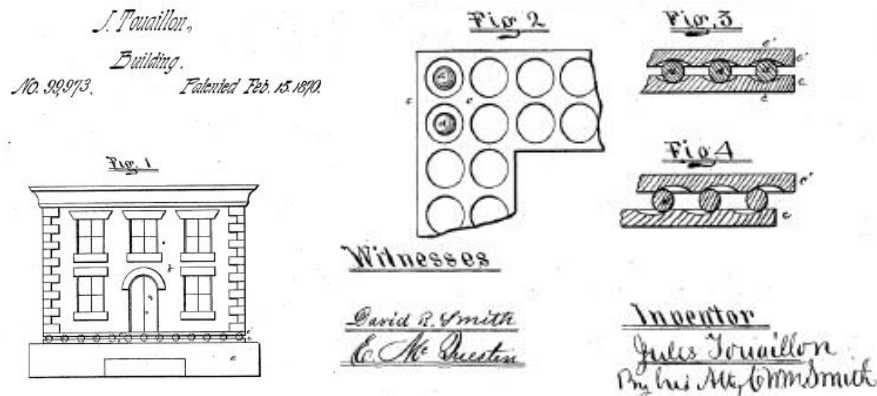
Snag: sliding bearings are rigid until a threshold force; this would lead to have an amplification of the seismic excitation on the artwork for small intensity earthquake.

Instead, rolling devices tend to have too little rolling resistance.

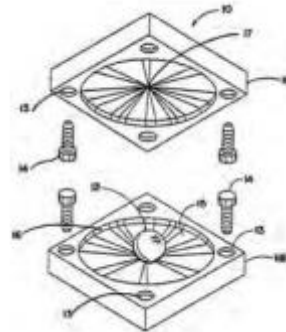
So the idea is to provide **rolling devices** with **damping** by using *rubber layers* on rollers or tracks, or using other *frictional materials*, or with some *auxiliary damping devices* (that work in parallel). For the **restoring force**, generally, it is used rubber or steel coil *spring*, or *no-flat rolling plane*.

The first patent about the rolling ball devices is by Touaillon in 1870 (see Tsai et al., 2010, *Earthq Eng & Eng Vib*, 9: 103-112). Other studies were performed by other researchers, as Schar (1910), Cummings (1930), Bakker (1935), Wu (1989), Kemeny (1997), Tsai (2006) ...

Touaillon, 1870



Ball-in-cone seismic isolation bearing, Kemeny, 1997.



Ball pendulum system «BPS», Tsai 2006.



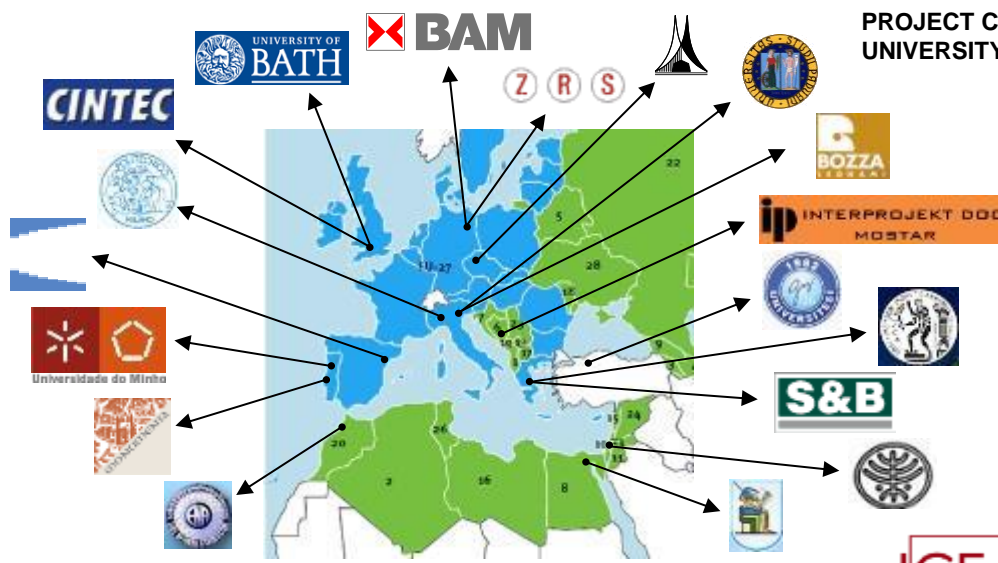
Seismic Risk Preparedness and Mitigation of Culture Heritage Sites

מוכנות והיערכות לסיכוני רעידות אדמה באתרי מורשת תרבות

ירושלים, יח' יט' בשבט, תשע"ד Israel, Jerusalem. 19-20 January 2014



NEW INTEGRATED KNOWLEDGE BASED APPROACHES TO THE PROTECTION OF CULTURAL HERITAGE FROM EARTHQUAKE INDUCED RISK - FP7-ENV-2009-



PROJECT COORDINATOR:
UNIVERSITY OF PADOVA

www.niker.eu



INGEGNERIA CIVILE,
EDILE E AMBIENTALE
CIVIL ARCHITECTURAL AND
ENVIRONMENTAL ENGINEERING



PROVACI
Formazione



Assisi (PG)



Aquileia (UD)



Development of sustainable techniques and methodologies for seismic protection, sustainable redevelopment and valorization of masonry buildings and archeological sites

Progetto **PROVACI**

Tecnologie per la **PRO**tezione sismica e la **VAL**orizzazione di
Complessi di Interesse culturale

IL PROGETTO DI RICERCA

Sviluppo di tecniche e metodologie integrate per la protezione sismica, la tutela, la
riqualificazione sostenibile e la valorizzazione di siti e strutture di interesse storico-artistico

9 aprile 2010

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- UNIVERSITA' DI NAPOLI "FEDERICO II"
- CONSORZIO CETMA
- SI.PRE s.r.l.
- **UNIVERSITA' DI PADOVA**
- C.R.A.C.A. Soc. Coop.
- NANOFAB s.c.a.r.l.



INGEGNERIA CIVILE,
EDILE E AMBIENTALE
CIVIL ARCHITECTURAL AND
ENVIRONMENTAL ENGINEERING



AGREEMENT AND PROJECTS WITH PUBLIC AUTHORITIES

- ARCUS – Seismic vulnerability analysis of Public National Museums and art objects, MIBAC
- Seismic vulnerability assessment of the regional road network – Regione Veneto
- Seismic vulnerability assessment by macro-classes of railway masonry bridges, RELUIS-RFI
- Vulnerability assessment of historic centres, public authorities and National Civil Defense
- Seismic analysis of relevant and strategic buildings (level 1 and level 2, OPCM 3274 e il DPCM 21/10/03), public authorities and National Civil Defense
- Activities related to L'Aquila (6/4/2009) and Emilia Romagna/Veneto/Lombardia (May 2012) earthquakes, Agreements with Regional Directorates of MIBAC, Municipalities, Technical Service for Reconstruction



THANK YOU!

SPEAKER: PROF. CLAUDIO MODENA
